

# **2018-2019 In-Class Test Solutions**

1.

A. elastic-perfectly-plastic

2.

C. 
$$
EI \frac{d^2y}{dx^2} = R_A x + M_B (x - a)^0 - P_C (x - b)
$$

# **Solution Q2**

Sectioning the beam after the last discontinuity (taking origin at left hand side of the beam) and drawing a Free Body Diagram of left hand side of section:



Taking moments about the section position (and applying Macauley's convention):

$$
M + P_C(x - b) = M_B(x - a)^0 + R_A x
$$
  
 
$$
\therefore M = R_A x + M_B(x - a)^0 - P_C(x - b)
$$

Substituting this into the deflection of beams equation:

$$
EI\frac{d^2y}{dx^2} = R_Ax + M_B\langle x-a\rangle^0 - P_C\langle x-b\rangle
$$



D. False

# **Solution Q3**

 $S_{range}$  and  $S_{amp}$  are labelled on the diagram the wrong way around.

4.

D. 162 MPa

# **Solution Q4**

Given  $\sigma$ <sub>z</sub> = 150 MPa and  $\tau$ <sub>z $\theta$ </sub> = 45 MPa

$$
C = \frac{\sigma_x + \sigma_y}{2} = \frac{150 + 0}{2} = 75 \text{ MPa}
$$
  

$$
R = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{z\theta}^2} = \sqrt{75^2 + 45^2} = 87.46 \text{ MPa}
$$
  

$$
\sigma_1 = C + R = 175 + 87.46 = 162.46 \approx 162 \text{ MPa}
$$

5.

D.  $0.8 \times 10^{-3}$  m

**Solution Q5**

$$
\delta = L\alpha \Delta T = 1 \times 16 \times 10^{-6} \times 50 = 8 \times 10^{-4} \text{ m}
$$

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6.

D. 
$$
y = \frac{1}{EI} \left( R_A \frac{x^3}{6} + P \frac{(x-4)^3}{6} - W \frac{(x-8)^4}{24} + Ax + B \right)
$$

**Solution Q6**

$$
EI\frac{d^2y}{dx^2} = R_Ax + P(x-4) - w\frac{(x-8)^2}{2}
$$

Integrating this gives:

$$
EI\frac{dy}{dx} = R_A \frac{x^2}{2} + P\frac{(x-4)^2}{2} - w\frac{(x-8)^3}{6} + A
$$

Integrating again gives:

$$
E I y = R_A \frac{x^3}{6} + P \frac{(x-4)^3}{6} - w \frac{(x-8)^4}{24} + Ax + B
$$

Rearranging this gives:

$$
y = \frac{1}{EI}\left(R_A\frac{x^3}{6} + P\frac{(x-4)^3}{6} - w\frac{(x-8)^4}{24} + Ax + B\right)
$$

7.

C. AB



B. 50 MPa

# **Solution Q8**

Torsional shear stress

$$
\tau = \frac{Tr}{J} = \frac{32 \times 500 \times 20 \times 10^{-3}}{\pi \times (40 \times 10^{-3})^4} = 39.8 \text{ MPa}
$$

Axial stress

$$
\sigma_a = \frac{F}{A} = \frac{75000}{\pi \times (20 \times 10^{-3})^2} = 59.7 \text{ MPa}
$$

Maximum shear stress is given by

$$
\tau_{max} = R = \sqrt{\left(\frac{\sigma_a}{2}\right)^2 + \tau^2} = \sqrt{29.85^2 + 39.8^2} = 49.75 \approx 50 \text{ MPa}
$$



B. 
$$
5 \times 10^{-3}
$$
 m

**Solution Q9**

$$
\sigma_{y(s,f)} = \frac{\sigma_y}{2} = 100 \text{ MPa}
$$

For a pressure vessel under internal pressure only

$$
\sigma_1 = \sigma_\theta
$$

$$
\sigma_2 = \sigma_a
$$

$$
\sigma_3 = 0
$$

Tresca yield criteria states yield will occur if

and

$$
\sigma_\theta = \frac{pr}{t}
$$

 $\sigma_1 - \sigma_3 = \sigma_\theta \geq \sigma_{\gamma(s,f)}$ 

Therefore, to avoid yield (including s.f.)

$$
t = \frac{pr}{\sigma_{y(s,f)}} = \frac{2 \times 10^6 \times 0.25}{100 \times 10^6} = 5 \times 10^{-3} \text{ m}
$$



## **Solution Q10**

 $K_{max} = 1.25\sigma\sqrt{\pi a}$ 

D. No

Therefore,

 $K_{I_c} = 1.25\sigma\sqrt{\pi a_c}$ (c = critical)

 $\left(\frac{K_{I_c}}{1.25\sigma}\right)$ 

 $\pi$ 

 $\overline{\mathbf{c}}$ 

 $\overline{\mathbf{c}}$ 

Rearranging,

 $= 0.001096 m = 1.096 mm$ 

 $=\frac{\left( \frac{55}{1.25 \times 750} \right)}{25}$ 

 $\pi$ 

 $a_c =$ 

Since the crack does not exceed the critical crack length, i.e. 0.75mm < 1.096mm, the component will not fracture under this load.

11.

B. is detrimental to fatigue life



D. 90.45mm

## **Solution Q12**

Moment Equilibrium:

$$
M = \int_A y\sigma dA = \int_{-d/2}^{d/2} y\sigma b dy = 2 \int_0^{d/2} y\sigma b dy
$$
  

$$
= 2 \left\{ \int_0^a y \left( \frac{\sigma_y}{a} y \right) b dy + \int_a^d y(\sigma_y) b dy \right\} = 2\sigma_y b \left\{ \int_0^a \frac{y^2}{a} dy + \int_a^d y dy \right\}
$$
  

$$
= 2\sigma_y b \left\{ \left[ \frac{y^3}{3a} \right]_0^a + \left[ \frac{y^2}{2} \right]_a^{d/2} \right\} = 2\sigma_y b \left\{ \frac{d^2}{8} - \frac{a^2}{6} \right\}
$$

$$
\therefore 300 \times 10^6 = 2 \times 275 \times 150 \times \left\{ \frac{200^2}{8} - \frac{a^2}{6} \right\}
$$

# :  $a = 90.45$ mm



B. 45 mm

**Solution Q13**

von Mises yield criteria for a plane stress case

$$
\sigma_1^2-\sigma_1\sigma_2+\sigma_2^2\geq\sigma_y^2
$$

For a shaft under pure torque, the Mohr's circle is centred on the origin and  $\sigma_1$  and  $\sigma_2$  will be the same magnitude, *k*, and also be the maximum allowable shear stress, therefore

$$
3k^2 = \sigma_y^2
$$

And the value of *k* will therefore be

$$
k = \frac{\sigma_y}{\sqrt{3}} = \frac{250 \times 10^6}{\sqrt{3}} = 144 \times 10^6
$$
 Pa

Therefore

$$
\tau = 144 \times 10^6 = \frac{Tr}{J} = \frac{2T}{\pi r^3}
$$

Rearranging

$$
r = \sqrt[3]{\frac{2 \times 20000}{144 \times 10^6 \pi}} = 0.0445 \text{ m} \approx 45 \text{ mm}
$$

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14.

C. -63 MPa

**Solution Q14** 

$$
\delta_{total} = 0 = \delta_{therm} + \delta_{mech} = L\alpha\Delta T + \frac{FL}{AE}
$$

Therefore

$$
-\alpha \Delta T = \frac{F}{AE}
$$

But  $\sigma = \frac{F}{A}$ , so

 $-\alpha\Delta TE = \sigma = -12 \times 10^{-6} \times 25 \times 210 \times 10^{9} = -63 \times 10^{6}$  Pa = -63 MPa

15.

**B.** y-planes



$$
A. \qquad 6 \text{ kN}
$$

## **Solution Q16**

Reaction forces will be present at positions B and E, namely  $R_B$  and  $R_E$  as shown below.



## Taking moments about position E:

 $R_B \times 4 = 7 \times 3 + 1.5 \times 2 \times 1$  $\therefore R_B = 6kN$ 

17.

#### 18.

# A. True

A. More conservative



**Solution Q19** 

$$
\delta_{total} = \delta_{initial} = \frac{F_{initial}L}{AE} = \delta_{therm} + \delta_{mech} = L\alpha\Delta T + \frac{FL}{AE}
$$

Therefore

$$
\frac{F_{initial}}{A} - \alpha \Delta TE = \frac{F}{A} = \frac{7500}{75 \times 10^{-6}} - 23 \times 10^{-6} \times 20 \times 70 \times 10^{9} = 67.8 \times 10^{6} \text{ Pa} = 68 \text{ MPa}
$$

20.

#### **Solution Q20**

Behaviour is assumed to be all elastic and therefore:

$$
\frac{M_y}{I} = \frac{\sigma_y}{y}
$$

where  $M_y$  is the moment required to cause yielding.

First yield will occur at  $y = \pm \frac{d}{2}$ , i.e. at the top and bottom edges:

$$
\therefore M_y = \frac{\sigma_y \times I}{y} = \frac{\sigma_y \times (\frac{bd^3}{12})}{\frac{d}{2}} = \frac{250 \times (\frac{150 \times 200^3}{12})}{\frac{200}{2}} = 250,000,000 Nmm = 250 kNm
$$

Since  $M > M_{\gamma}$ , yielding does occur.