

# 2018-2019 In-Class Test Solutions

1.

A. elastic-perfectly-plastic

2.

C. 
$$EI\frac{d^2y}{dx^2} = R_A x + M_B \langle x - a \rangle^0 - P_C \langle x - b \rangle$$

#### Solution Q2

Sectioning the beam after the last discontinuity (taking origin at left hand side of the beam) and drawing a Free Body Diagram of left hand side of section:



Taking moments about the section position (and applying Macauley's convention):

$$M + P_C \langle x - b \rangle = M_B \langle x - a \rangle^0 + R_A x$$
  
$$\therefore M = R_A x + M_B \langle x - a \rangle^0 - P_C \langle x - b \rangle$$

Substituting this into the deflection of beams equation:

$$EI\frac{d^2y}{dx^2} = R_A x + M_B \langle x - a \rangle^0 - P_C \langle x - b \rangle$$



## Solution Q3

 $S_{range}$  and  $S_{amp}$  are labelled on the diagram the wrong way around.

4.

## Solution Q4

Given  $\sigma_{\!z}=150~{\rm MPa}$  and  $\tau_{z\theta}=45~{\rm MPa}$ 

$$C = \frac{\sigma_x + \sigma_y}{2} = \frac{150 + 0}{2} = 75 \text{ MPa}$$
$$R = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{z\theta}^2} = \sqrt{75^2 + 45^2} = 87.46 \text{ MPa}$$
$$\sigma_1 = C + R = 175 + 87.46 = 162.46 \approx 162 \text{ MPa}$$

5.

Solution Q5

$$\delta = L\alpha\Delta T = 1 \times 16 \times 10^{-6} \times 50 = \mathbf{8} \times \mathbf{10}^{-4} \mathbf{m}$$

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6.

D. 
$$y = \frac{1}{EI} \left( R_A \frac{x^3}{6} + P \frac{(x-4)^3}{6} - w \frac{(x-8)^4}{24} + Ax + B \right)$$

Solution Q6

$$EI\frac{d^2y}{dx^2} = R_A x + P\langle x - 4 \rangle - w\frac{\langle x - 8 \rangle^2}{2}$$

Integrating this gives:

$$EI\frac{dy}{dx} = R_A \frac{x^2}{2} + P\frac{\langle x - 4 \rangle^2}{2} - w\frac{\langle x - 8 \rangle^3}{6} + A$$

Integrating again gives:

$$EIy = R_A \frac{x^3}{6} + P \frac{\langle x - 4 \rangle^3}{6} - w \frac{\langle x - 8 \rangle^4}{24} + Ax + B$$

Rearranging this gives:

$$y = \frac{1}{EI} \left( R_A \frac{x^3}{6} + P \frac{\langle x - 4 \rangle^3}{6} - w \frac{\langle x - 8 \rangle^4}{24} + Ax + B \right)$$

7.

C. AB



B. 50 MPa

## Solution Q8

Torsional shear stress

$$\tau = \frac{Tr}{J} = \frac{32 \times 500 \times 20 \times 10^{-3}}{\pi \times (40 \times 10^{-3})^4} = 39.8 \text{ MPa}$$

Axial stress

$$\sigma_a = \frac{F}{A} = \frac{75000}{\pi \times (20 \times 10^{-3})^2} = 59.7 \text{ MPa}$$

Maximum shear stress is given by

$$\tau_{max} = R = \sqrt{\left(\frac{\sigma_a}{2}\right)^2 + \tau^2} = \sqrt{29.85^2 + 39.8^2} = 49.75 \approx 50 \text{ MPa}$$



Solution Q9

$$\sigma_{y(s.f)} = \frac{\sigma_y}{2} = 100 \text{ MPa}$$

For a pressure vessel under internal pressure only

$$\sigma_1 = \sigma_\theta$$
$$\sigma_2 = \sigma_a$$
$$\sigma_3 = 0$$

$$\sigma_1 - \sigma_3 = \sigma_\theta \ge \sigma_{y(s.f)}$$

and

$$\sigma_{\theta} = \frac{pr}{t}$$

Therefore, to avoid yield (including s.f.)

$$t = \frac{pr}{\sigma_{y(s.f)}} = \frac{2 \times 10^6 \times 0.25}{100 \times 10^6} = 5 \times 10^{-3} \text{ m}$$



#### Solution Q10

 $K_{max} = 1.25\sigma\sqrt{\pi a}$ 

No

D.

Therefore,

 $K_{I_c} = 1.25\sigma\sqrt{\pi a_c}$ (c = critical)

Rearranging,

= 0.001096m = 1.096mm

 $a_c = \frac{\left(\frac{K_{I_c}}{1.25\sigma}\right)^2}{\pi}$ 

 $=\frac{\left(\frac{55}{1.25\times750}\right)^2}{\left(\frac{55}{1.25\times750}\right)^2}$ 

Since the crack does not exceed the critical crack length, i.e. 0.75mm < 1.096mm, the component **will not fracture** under this load.

11.

B. is detrimental to fatigue life



D. 90.45mm

#### Solution Q12

Moment Equilibrium:

$$M = \int_{A} y\sigma dA = \int_{-d/2}^{d/2} y\sigma bdy = 2 \int_{0}^{d/2} y\sigma bdy$$
$$= 2 \left\{ \int_{0}^{a} y \left(\frac{\sigma_{y}}{a}y\right) bdy + \int_{a}^{d/2} y(\sigma_{y}) bdy \right\} = 2\sigma_{y}b \left\{ \int_{0}^{a} \frac{y^{2}}{a} dy + \int_{a}^{d/2} ydy \right\}$$
$$= 2\sigma_{y}b \left\{ \left[\frac{y^{3}}{3a}\right]_{0}^{a} + \left[\frac{y^{2}}{2}\right]_{a}^{d/2} \right\} = 2\sigma_{y}b \left\{ \frac{d^{2}}{8} - \frac{a^{2}}{6} \right\}$$

$$\therefore 300 \times 10^6 = 2 \times 275 \times 150 \times \left\{ \frac{200^2}{8} - \frac{a^2}{6} \right\}$$

# $\therefore a = 90.45mm$



B. 45 mm

Solution Q13

von Mises yield criteria for a plane stress case

$$\sigma_1^2 - \sigma_1 \sigma_2 + \sigma_2^2 \ge \sigma_y^2$$

For a shaft under pure torque, the Mohr's circle is centred on the origin and  $\sigma_1$  and  $\sigma_2$  will be the same magnitude, k, and also be the maximum allowable shear stress, therefore

$$3k^2 = \sigma_y^2$$

And the value of k will therefore be

$$k = \frac{\sigma_y}{\sqrt{3}} = \frac{250 \times 10^6}{\sqrt{3}} = 144 \times 10^6 \text{ Pa}$$

Therefore

$$\tau = 144 \times 10^6 = \frac{Tr}{J} = \frac{2T}{\pi r^3}$$

Rearranging

$$r = \sqrt[3]{\frac{2 \times 20000}{144 \times 10^6 \pi}} = 0.0445 \text{ m} \approx 45 \text{ mm}$$



$$\delta_{total} = 0 = \delta_{therm} + \delta_{mech} = L\alpha\Delta T + \frac{FL}{AE}$$

– 63 MPa

C.

Therefore

$$-\alpha\Delta T = \frac{F}{AE}$$

But  $\sigma = \frac{F}{A}$ , so

 $-\alpha \Delta TE = \sigma = -12 \times 10^{-6} \times 25 \times 210 \times 10^{9} = -63 \times 10^{6} \text{ Pa} = -63 \text{ MPa}$ 

15.

B. y-planes



# Solution Q16

Reaction forces will be present at positions B and E, namely  $R_B$  and  $R_E$  as shown below.



#### Taking moments about position E:

 $R_B \times 4 = 7 \times 3 + 1.5 \times 2 \times 1$  $\therefore R_B = \mathbf{6kN}$ 

17.

A. True

A. More conservative



#### Solution Q19

$$\delta_{total} = \delta_{initial} = \frac{F_{initial}L}{AE} = \delta_{therm} + \delta_{mech} = L\alpha\Delta T + \frac{FL}{AE}$$

Therefore

$$\frac{F_{initial}}{A} - \alpha \Delta TE = \frac{F}{A} = \frac{7500}{75 \times 10^{-6}} - 23 \times 10^{-6} \times 20 \times 70 \times 10^9 = 67.8 \times 10^6 \text{ Pa} = 68 \text{ MPa}$$

20.

#### Solution Q20

Behaviour is assumed to be all elastic and therefore:

$$\frac{M_y}{I} = \frac{\sigma_y}{y}$$

where  $M_y$  is the moment required to cause yielding.

First yield will occur at  $y = \pm \frac{d}{2}$ , i.e. at the top and bottom edges:

$$\therefore M_y = \frac{\sigma_y \times I}{y} = \frac{\sigma_y \times \left(\frac{bd^3}{12}\right)}{\frac{d}{2}} = \frac{250 \times \left(\frac{150 \times 200^3}{12}\right)}{\frac{200}{2}} = 250,000,000Nmm = 250kNm$$

Since  $M > M_y$ , yielding does occur.